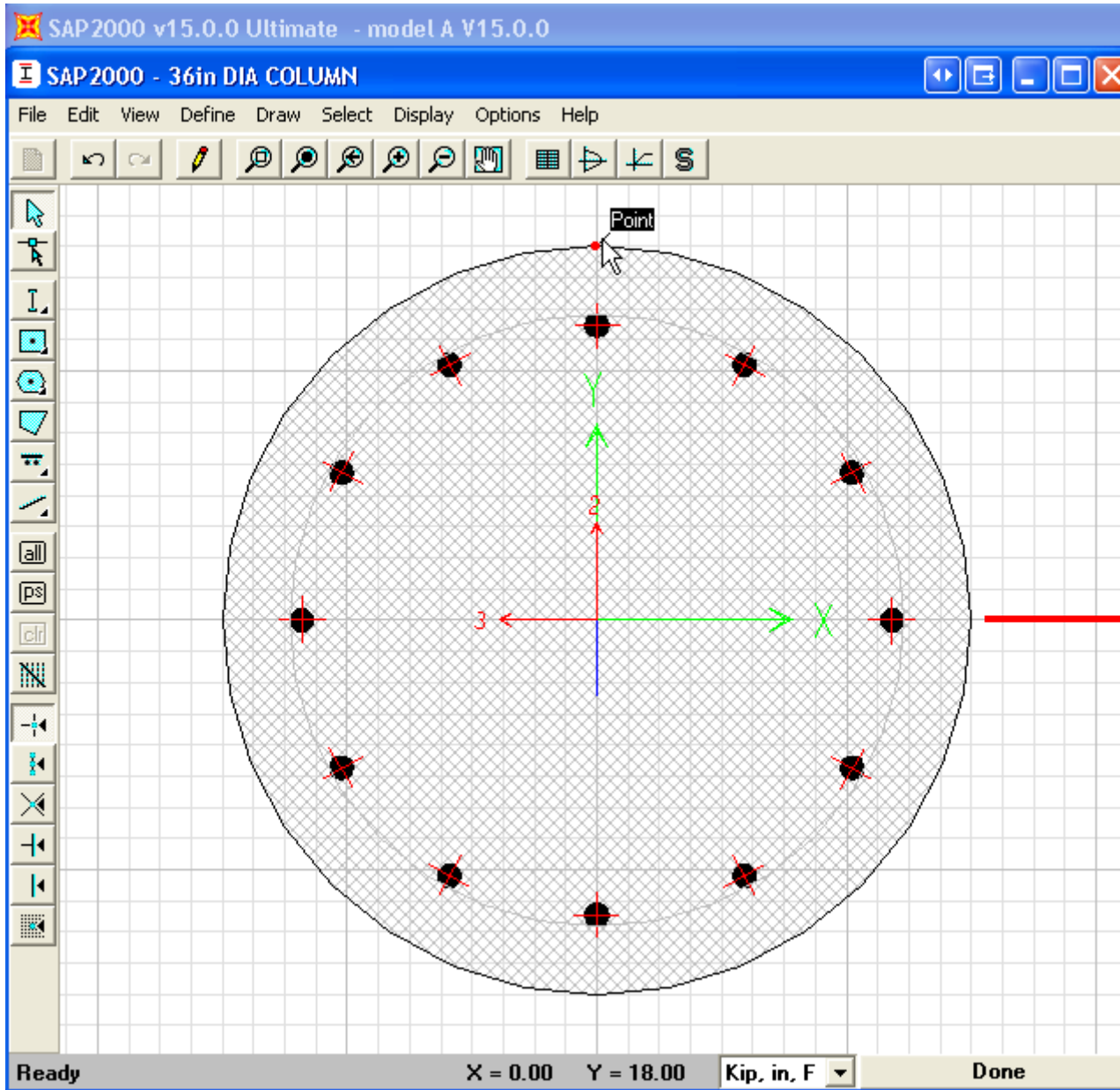


Moment curvature and cracked moment of inertia

- The purpose of this test problem is to explain the meaning of various parameters reported for the moment curvature curve and illustrate how is the cracked moment of inertia calculated.
- The moment curvature analysis is performed for circular column section described in detail in Example 2 of Reference [1].

Cross-Section



Angle = 0
(angle is
measured from
from negative 3
axis)

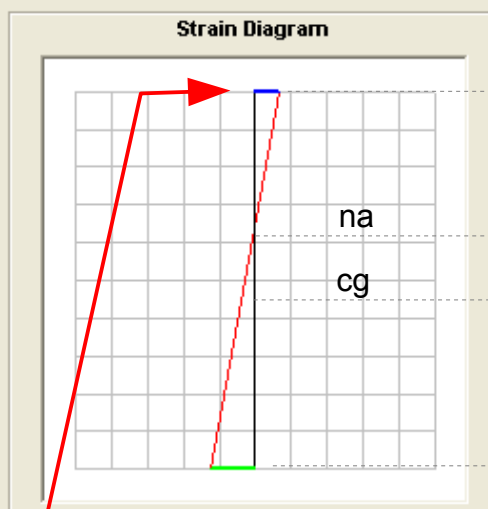
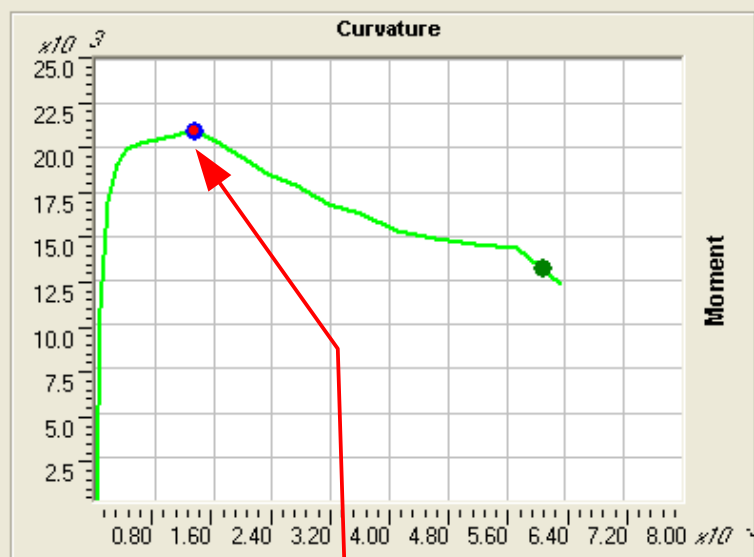
Explanation of Moment-Curvature Output Parameters

Exact Integration Curve

Phi-Conc, M-Conc

- Phi-Conc is the curvature and M-Conc is the moment for which ultimate concrete strain is reached in extreme compressive fiber

Moment Curvature Curve (Limits: P(comp.) = -4117.784, P(ten.) = 720)



Concrete Strain -0.0151
Steel Strain 0.0283
Neutral Axis 6.7872

Select Type of Graph: Moment-Curvature
Specify Scales/Headings...
(1.351E-03 , 20931.10)

☒ Plot Exact-Integration Curve ☒ Show Numerical Results for Exact-Integration Curve
☐ Plot 3x3 Fiber Model Curve ☐ Show Numerical Results for Fiber Model Curve

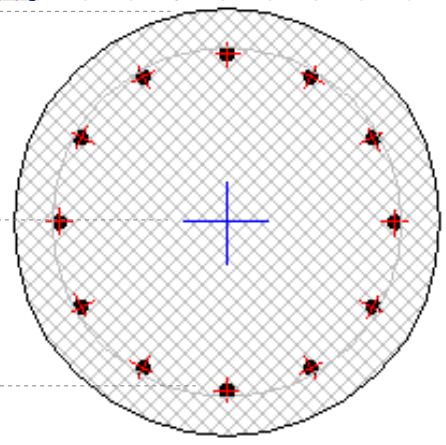
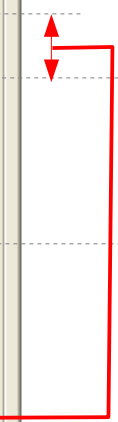
☐ Caltrans Idealized Model
P [Tension +ve] -1000
Max Curvature 0
No. of Points 20
Angle (Deg) 0

Phi-Conc = .00134155 M-Conc = 20949.858
Phi-Steel = .00610152 M-Steel = 13114.275

Analysis Control
☐ Concrete Failure (Lowest Ultimate Strain)
☒ Concrete Failure (Highest Ultimate Strain)
☒ First Rebar/Tendon Failure
☐ User Defined Curvature

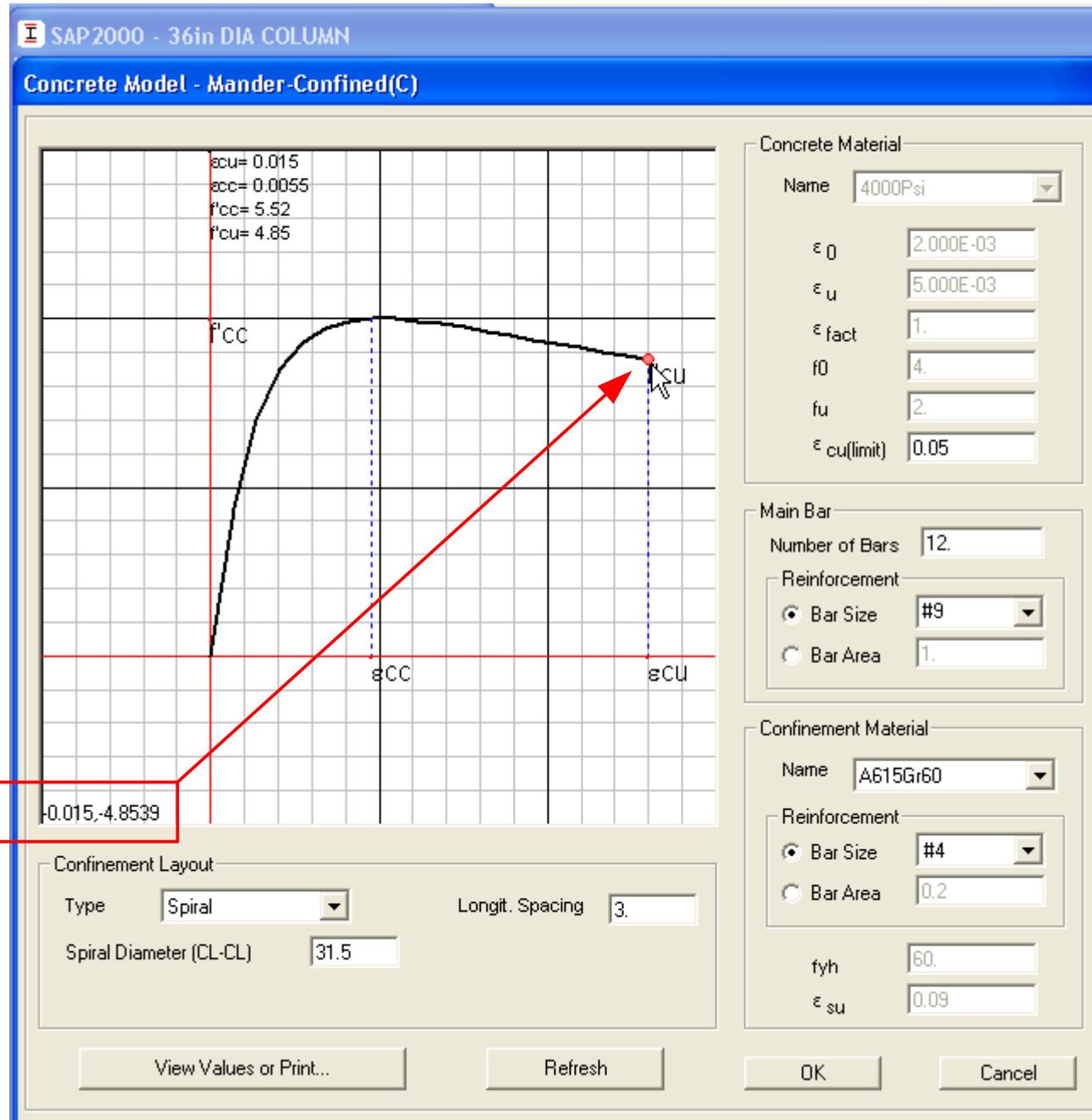
Details... Contour...
Refresh Done

Curves
New Curve



Selected Curve Color
Click to:
Add Curve
Delete Curve

To obtain the concrete stress-strain curve shown below, right click on the material region in Section Designer and select "C Model":

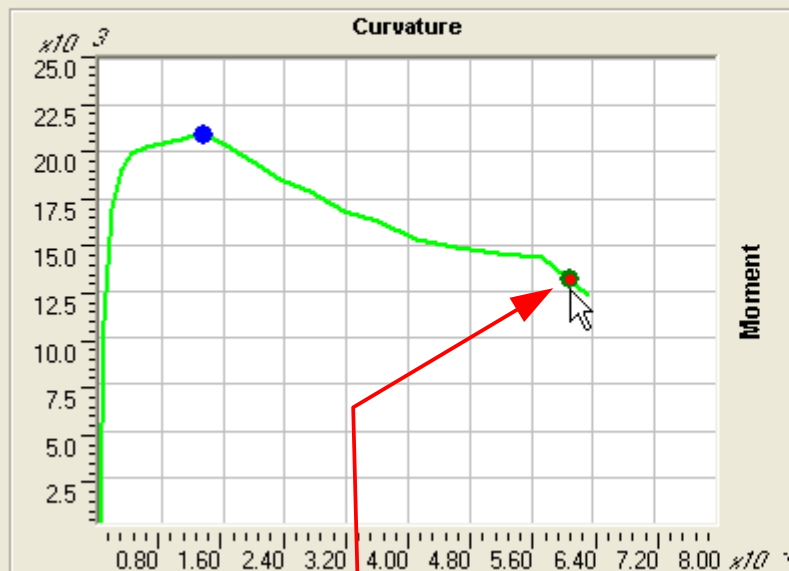


Exact Integration Curve

Phi-Steel, M-Steel

- Phi-Steel is the curvature and M-Steel is the moment for which failure reinforcement strain/stress is reached in the any reinforcement bar

Moment Curvature Curve (Limits: P(comp.) = -4117.784, P(ten.) = 720)



Select Type of Graph

Moment-Curvature

Specify Scales/Headings...

(6.104E-03 , 13106.45)

☒ Plot Exact-Integration Curve☐ Plot 3x3 Fiber Model Curve☒ Show Numerical Results for Exact-Integration Curve☐ Show Numerical Results for Fiber Model Curve☐ Caltrans Idealized Model

P [Tension +ve] -1000

Max Curvature 0

Phi-Conc = .00134155

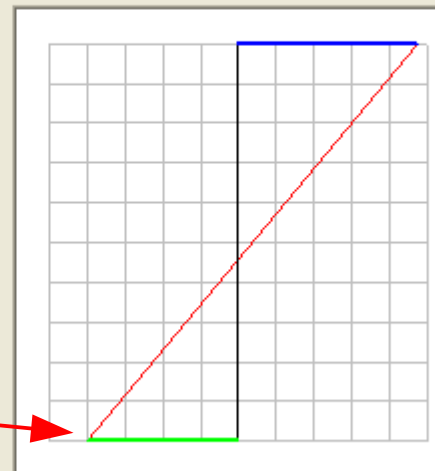
Phi-Steel = .00610152

No. of Points 20

Angle (Deg) 0.

M-Conc = 20949.858

M-Steel = 13114.275

Strain Diagram

Concrete Strain -0.1064

Steel Strain 0.09

Neutral Axis 0.5775

Analysis Control

☐ Concrete Failure (Lowest Ultimate Strain)☒ Concrete Failure (Highest Ultimate Strain)☒ First Rebar/Tendon Failure☐ User Defined Curvature

Details...

Contour...

Refresh

Done

Curves

New Curve

Selected Curve Color

Click to:

Add Curve

Delete Curve

Nonlinear Material Data

Edit

Material Name: A615Gr60 Material Type: Rebar

Hysteresis Type: Kinematic Drucker-Prager Parameters: Friction Angle: Dilatational Angle: Units: Kip, in, F

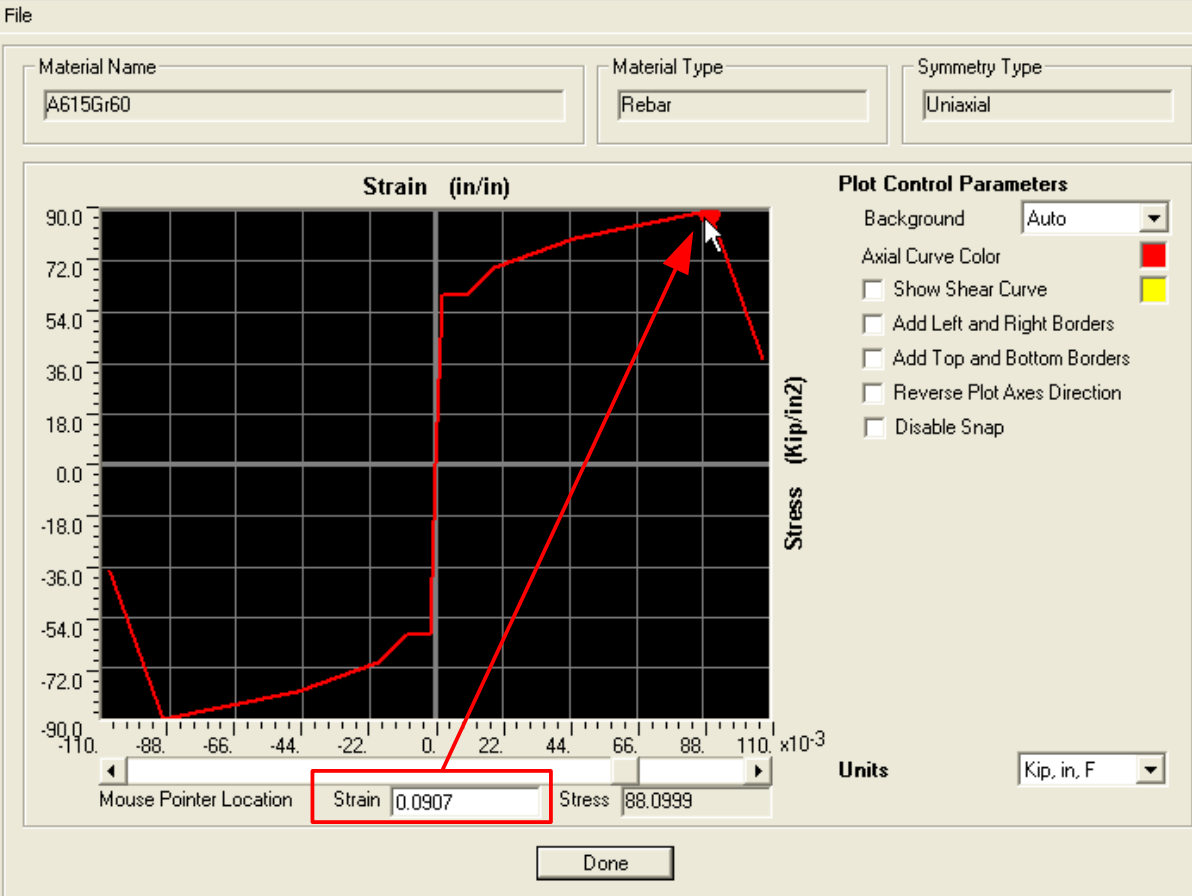
Stress-Strain Curve Definition Options:
☒ Parametric Simple Convert To User Defined
☐ User Defined

Parametric Strain Data:
Strain At Onset of Strain Hardening: 0.01
Ultimate Strain Capacity: 0.09
Final Slope (Multiplier on E): -0.1
☐ Use Caltrans Default Controlling Strain Values (Bar Size Dependent)

Show Stress-Strain Plot...

OK Cancel

Material Stress-Strain Curve Plot



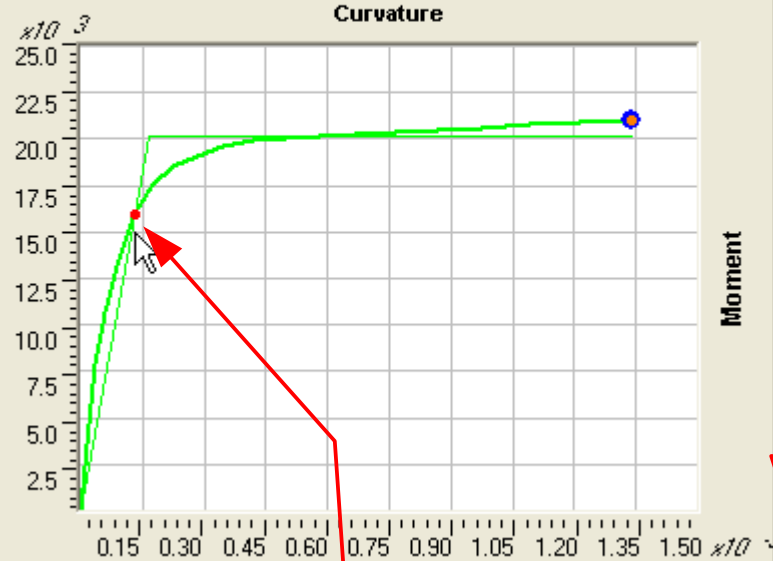
The ultimate steel strain of 0.0907 corresponds to the steel strain shown on the previous page

Caltrans Model

Φ -yield(initial), M-yield

- Φ -yield(initial) is the curvature and M-yield is the moment for the the first rebar in the cross-section starts to yield

Moment Curvature Curve (Limits: P(comp.) = -4117.784, P(ten.) = 720)



Select Type of Graph: Moment-Curvature
Specify Scales/Headings...: [1.315E-04 , 15919.87]

☒ Plot Exact-Integration Curve ☒ Show Numerical Results for Exact-Integration Curve
☐ Plot 3x3 Fiber Model Curve ☐ Show Numerical Results for Fiber Model Curve

☒ Caltrans Idealized Model
P [Tension +ve]: -1000
Max Curvature: 1.342E-03
Phi-Conc = .00134155
Phi-Steel = N/A
Phi-yield(Initial) = .00013205
Phi-yield(Idealized) = .00016702
ICrack = 33487.397
No. of Points: 20
Angle (Deg): 0
Mmax = 20966.92
M-Conc = 20966.92
M-Steel = N/A
Mp = 20163

Strain Diagram



Concrete Strain: -2.175E-03
Steel Strain: 2.058E-03
Neutral Axis: 1.4338

Analysis Control

☐ Concrete Failure (Lowest Ultimate Strain)
☒ Concrete Failure (Highest Ultimate Strain)
☒ First Rebar/Tendon Failure
☐ User Defined Curvature

Details...

Contour...

Refresh

Done

Curves

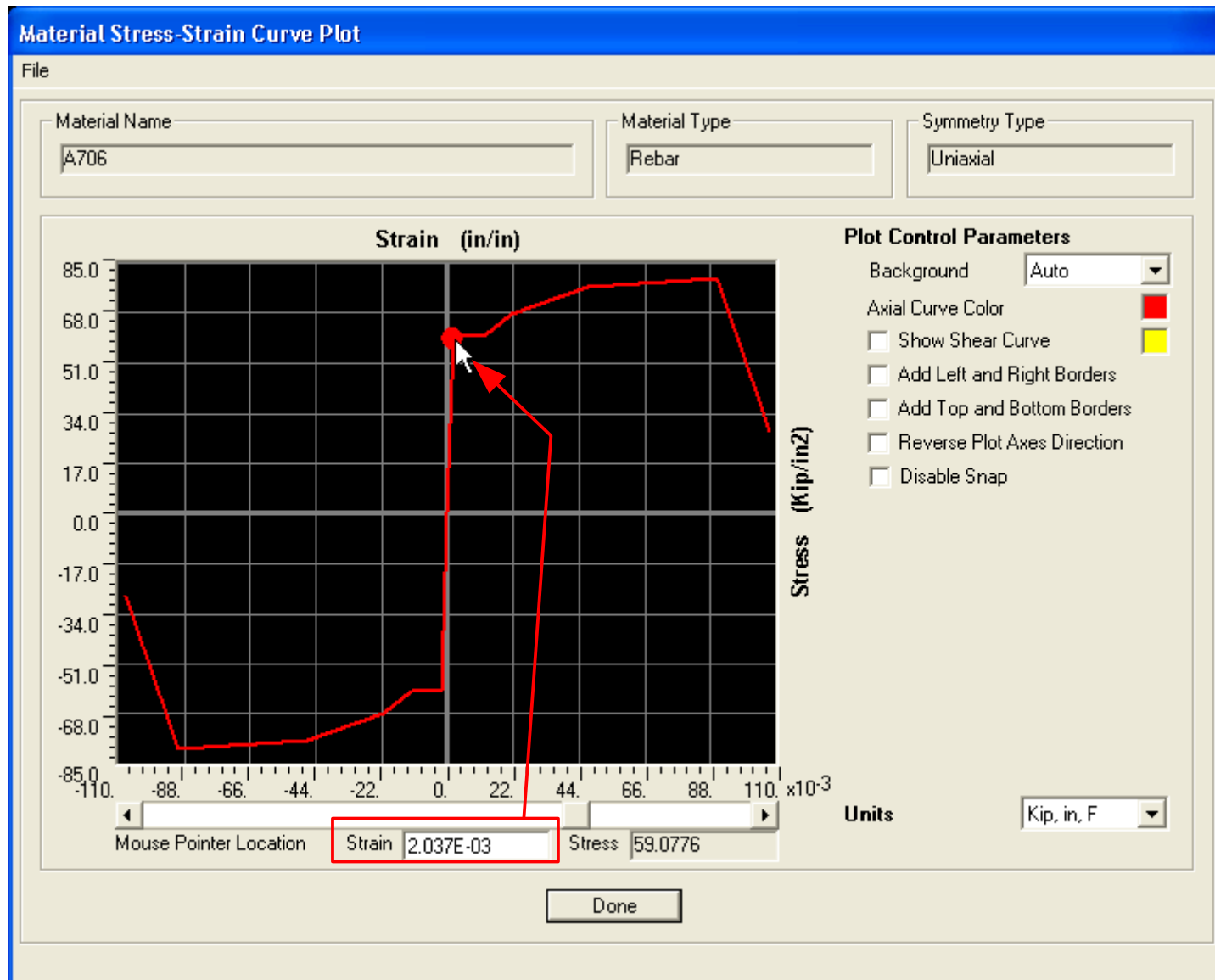
New Curve

Selected Curve Color: [Black]

Click to:

Add Curve

Delete Curve



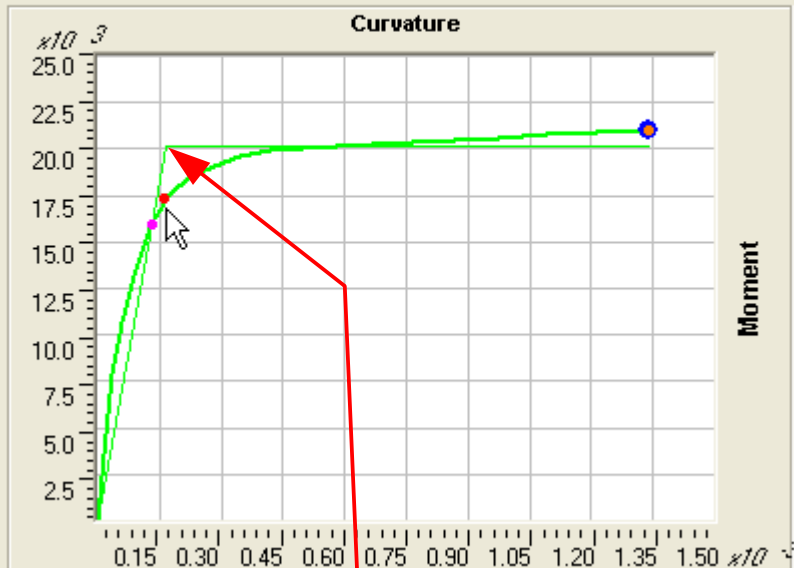
The yield steel strain of 0.002037 corresponds to the steel strain shown on the previous page

Caltrans Model

$\Phi_{\text{yield(idealized)}}$, M_p

- $\Phi_{\text{yield(idealized)}}$ is the curvature and M_p is the moment for the initial yield on the Caltrans idealized moment curvature curve.
- The idealized curve is obtained as per Caltrans Seismic Design Criteria [2].

Moment Curvature Curve (Limits: P(comp.) = -4117.784, P(ten.) = 720)



Select Type of Graph

Moment-Curvature

Specify Scales/Headings...

(1.656E-04 , 17256.51)

☒ Plot Exact-Integration Curve

☐ Plot 3x3 Fiber Model Curve



☒ Show Numerical Results for Exact-Integration Curve



☐ Show Numerical Results for Fiber Model Curve

☒ Caltrans Idealized Model

P [Tension +ve] -1000

Max Curvature 1.342E-03

Phi-Conc = .00134155

Phi-Steel = N/A

Phi-yield(Initial) = .00013205

Phi-yield(Idealized) = .00016702

ICrack = 33487.397

No. of Points 20

Angle (Deg) 0.

Mmax = 20966.92

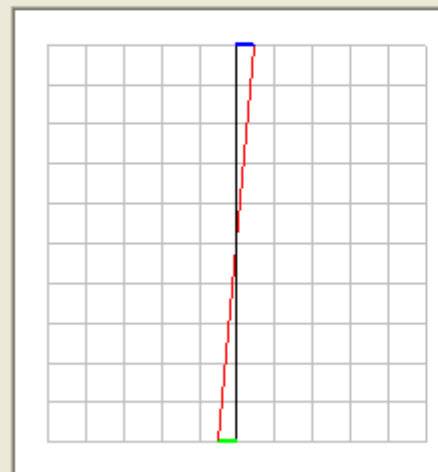
M-Conc = 20966.92

M-Steel = N/A

M-yield = 15941.855

Mp = 20163

Strain Diagram



Concrete Strain -2.576E-03

Steel Strain 2.753E-03

Neutral Axis 2.3917

Analysis Control

☐ Concrete Failure (Lowest Ultimate Strain)

☒ Concrete Failure (Highest Ultimate Strain)

☒ First Rebar/Tendon Failure

☐ User Defined Curvature

Details...

Contour...

Refresh

Done

Curves

New Curve

Selected Curve Color

Click to:

Add Curve

Delete Curve

The idealized M_p on the previous page is determined as specified in Caltrans Seismic Design Criteria, June 2006, Version 1.4:

3.3 Plastic Moment Capacity for Ductile Concrete Members

3.3.1 Moment Curvature ($M-\phi$) Analysis

The plastic moment capacity of all ductile concrete members shall be calculated by $M-\phi$ analysis based on expected material properties. Moment curvature analysis derives the curvatures associated with a range of moments for a cross section based on the principles of strain compatibility and equilibrium of forces. The $M-\phi$ curve can be idealized with an elastic perfectly plastic response to estimate the plastic moment capacity of a member's cross section. The elastic portion of the idealized curve should pass through the point marking the first reinforcing bar yield. The idealized plastic moment capacity is obtained by balancing the areas between the actual and the idealized $M-\phi$ curves beyond the first reinforcing bar yield point, see Figure 3.7 [4].

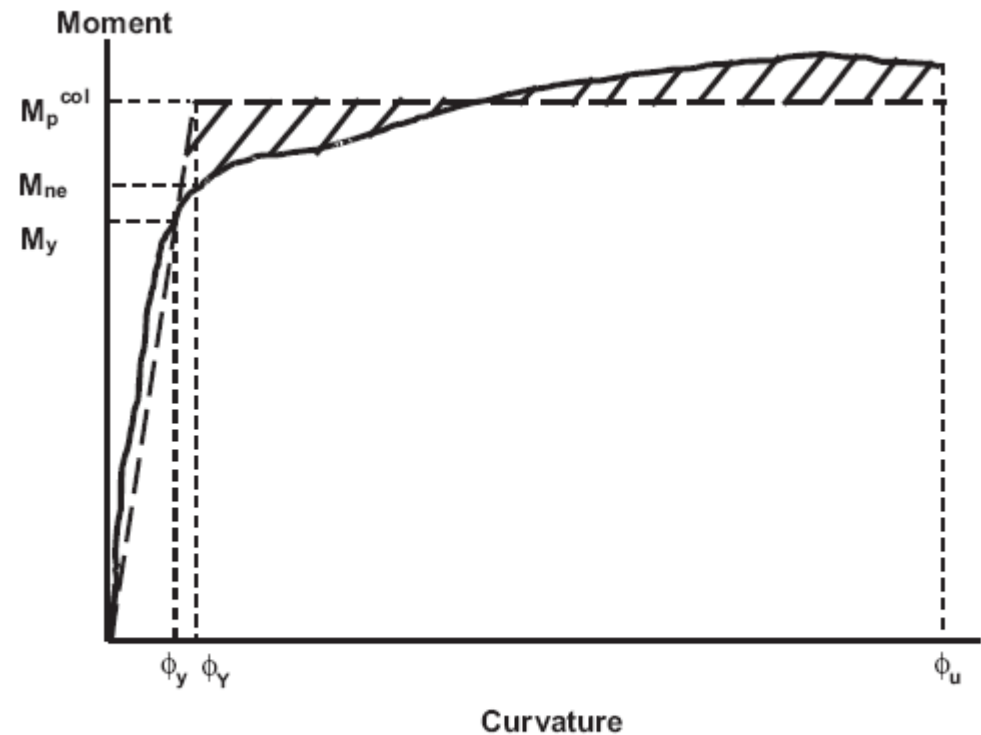


Figure 3.7 Moment Curvature Curve

Caltrans Model Icrack

Icrack is calculated as follows:

Basic equation:

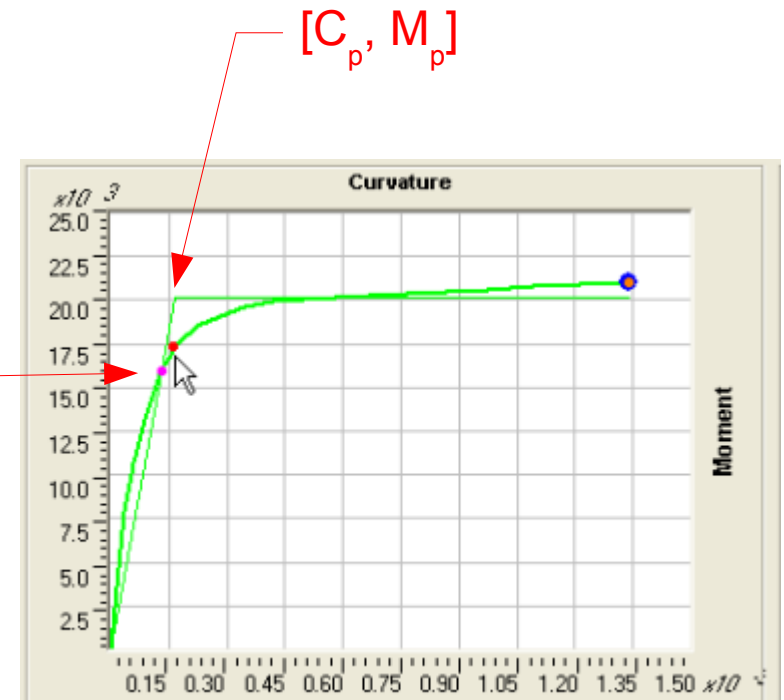
$$\phi = \frac{M}{EI}$$

Caltrans idealized moment-curvature equations:

$$I_{crack} = \frac{M_p}{C_p E} = \frac{20163 \text{ kip-in}}{(0.00016702 \text{ rad/in})(3604 \text{ ksi})} = 33483 \text{ in}^4$$

$$I_{crack} = \frac{M_y}{C_y E}$$

$[C_y, M_y]$



- Compare $I_{crack} = 33483 \text{ in}^4$ with gross moment of inertia of 81394 in^4

Property Data

Section Name: 36in DIA COLUMN

Properties

Cross-section (axial) area	1011.3482	Section modulus about 3 axis	4521.9179
Torsional constant	162777.68	Section modulus about 2 axis	4521.9179
Moment of Inertia about 3 axis	81394.52	Plastic modulus about 3 axis	7701.2932
Moment of Inertia about 2 axis	81394.52	Plastic modulus about 2 axis	7701.2932
Shear area in 2 direction	913.0094	Radius of Gyration about 3 axis	8.9711
Shear area in 3 direction	913.0094	Radius of Gyration about 2 axis	8.9711

OK

References

- [1] Robert Mathews: Moment Curvature, 2001.
http://www.structsource.com/pdf/Momcurv_web.pdf
- [2] Caltrans Seismic Design Criteria, Version 1.4
<http://www.dot.ca.gov/hq/esc/techpubs/manual/othermanual/other-engineering-manual/seismic-design-criteria/sdc.html>